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INFRASTRUCTURE NEEDS STUDY
FOR THE SUMMIT CITY AND
AND RIDDLE ROAD AREAS

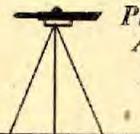
FOR

CITY OF SHASTA LAKE

**(REPORT PREPARED WITH COMMUNITY DEVELOPMENT
BLOCK GRANT PLANNING/TECHNICAL ASSISTANCE FUNDS)**

MAY 1997

PACE
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*Professional
Associated
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Engineers*

REDDING, CALIFORNIA

Incorporated

ENGINEERING REPORT
INFRASTRUCTURE NEEDS STUDY
FOR THE SUMMIT CITY AND
AND RIDDLE ROAD AREAS

FOR

CITY OF SHASTA LAKE
P.O. Box 777
SHASTA LAKE, CA 96019

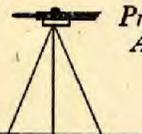
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MAY 1997

JOB No. 110.24



PACE
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June 3, 1997

110.24

Mr. Alan Harvey
City Manager
City of Shasta Lake
P.O. Box 777
Shasta Lake, CA 96019

Dear Mr. Harvey:

We are pleased to present our Engineering Report entitled:

**INFRASTRUCTURE NEEDS STUDY
FOR THE SUMMIT CITY AND RIDDLE ROAD AREAS**

This report contains the results of the City's surface water sampling survey, the results of the mail out questionnaire regarding sewage disposal and storm water drainage problems, and a preliminary storm drainage plan for the two study areas. It also includes preliminary layouts and preliminary cost estimates of sewage collection systems and storm drain improvements in both study areas.

A summary of the Income Survey that was conducted as part of this study is included in Appendix A.

PACE Engineering is very pleased to have performed this study for the City. At your convenience, we will be pleased to meet with you to discuss this report in detail.

Very truly yours,

Samuel L. Smith
Principal Engineer

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enclosures
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EXECUTIVE SUMMARY

SEWAGE DISPOSAL INFRASTRUCTURE NEEDS

SUMMIT CITY AREA: Based on the results of the Phase I—Initial Study and the property owner questionnaire, there appears to be a number of problems associated with on-site sewage disposal in the high density area of Summit City which consists primarily of single family residences on small lots and some commercial development. However, in order to qualify for grant funding it will probably be necessary to dye a number of the on-site systems during wet weather conditions to adequately document system failures. The timing of such dye testing is critical because most of the problems appear to be related to high groundwater and flooding associated with wet winter and springtime conditions.

If adequate documentation of pollution and/or a public health hazard can be obtained, it may be possible to secure up to 75 percent grant funding from Rural Development for the sewer system capital improvements costs. With a 75 percent grant, it is estimated that the total monthly cost for sewer service in the Summit City area would be about \$37 per month per household equivalent in June 1997 dollars. Without grant funding, the total monthly cost would be about \$95 per month.

Recommended Follow-up Action:

1. Set up a citizens advisory committee of Summit City residents to help develop a plan of action and to provide a link for communication between area residents and City staff.
2. Consult with the Shasta County Environmental Health Department and devise a plan for wet weather dye testing of septic systems in the Summit City area to ascertain, if a public health hazard does exist due to failing septic systems.

Typically a significant problem exists if 25 to 30 percent of the septic systems in an area are failing.

3. If a public health hazard is documented that can not be mitigated by reconstruction of septic systems to comply with current standards, then the City should consider adopting a building moratorium on the affected area. Input from the Shasta County Environmental Health Department should be obtained when evaluating the significance of the problem and determining whether a building moratorium is appropriate.
4. If a building moratorium is adopted, then the opportunity to obtain grant and loan funding will be greatly increased. The next step will be to prepare a Preliminary Engineering Report to develop the best apparent alternative to mitigate the public health hazard and apply to Rural Development, Community Development Block Grant and other grant/loan agencies for project funding.

RIDDLE ROAD AREA: Based upon the results of the property owner questionnaire, there appears to be a number of dwellings in the Riddle Road area that are having problems with their on-site sewage disposal systems. Since the lots in the Riddle Road area are one-half acre or larger, it may be possible to solve their septic system problems by installing alternative systems such as mounded systems. However, if the alternative systems are not feasible then the area can be connected to the city sewer system via a sewage collection system as shown on Figure 9. In order for a sewage collection system for the Riddle Road area to be grant eligible, adequate documentation of pollution and/or a public health hazard will need to be obtained as outlined above for the Summit City area.

If a 75 percent grant could be obtained for the sewer system capital improvement costs in the Riddle Road area then, it is estimated that the total monthly cost for sewer service in the Riddle Road area would be about \$44 per month per household equivalent in June 1997 dollars. Without grant funding the total monthly cost would be about \$125 per month.

Recommended Follow-up Action: Due to the relatively small size of the Riddle Road area, it is recommended that each property owner be contacted to determine the level of their problem and the interest in being connected to the city sewer system. If there is sufficient interest existing, then follow the same steps outlined above for Summit City area.

DRAINAGE INFRASTRUCTURE NEEDS

SUMMIT CITY AREA: The drainage structures along Churn Creek that are maintained by the city of Shasta Lake are adequate to carry the 100-year storm runoff with two exceptions. The 30-inch pipe and the 2-48" pipes crossing Lake Blvd. approximately 0.6 mile north of Shasta Dam Blvd. are too small. The cost to increase the capacity at these locations is approximately \$25,500.

The highest priority should be given to the Summit City area east of Hill Blvd. and between Rose Avenue and Spruce Avenue.

Most of the drainage systems in this area are inadequate to carry the design storm. One solution is to construct a bypass north of Rose Avenue from Sacramento Street westerly to Churn Creek. The approximate cost is \$245,000. In the area from Rose Avenue to Ranchera Road, plastic pipes could be inserted into the existing pipes as they rust out. The void between the pipes could be grouted with a cement slurry.

Another solution is to replace the existing pipes from Rose Avenue to Ranchera Road with larger pipes capable of carrying the design flow. The existing ditches would also need to be made deeper to carry the larger flow. The cost for this alternative is approximately \$260,000, not including right-of-way costs, if necessary.

Recommended Follow-up Action:

1. Have the City maintenance crews inspect the condition of the existing culverts and drainage ditches to schedule and complete the necessary cleaning to restore their hydraulic capacity.
2. Replace the plugged 12-inch culverts near Beacon Street and Pine Avenue with 18-inch culverts and concrete inlet structures.
3. Prepare a topographic map and preliminary design for the Rose Avenue bypass to confirm the location and right-of-way needs and fine tune the construction cost estimate. The estimate cost of this preliminary work is estimated at \$4,000.
4. Apply for a Community Development Block Grant or other funding to construct the Rose Avenue bypass.

RIDDLE ROAD AREA: The culverts across Indian Avenue and the small bridge across Arrowhead Avenue in the Riddle Road area are adequate for the existing gravel road conditions. However, they should be replaced with two 48-inch culverts if and when the roads are constructed to City standards and accepted by the City for maintenance.

INTRODUCTION

PURPOSE AND SCOPE OF STUDY

The initial purpose of this study was to determine if water quality problems and/or public health hazards exist in the unsewered Summit City and Riddle Road areas as a result of the existing on-site sewage disposal systems. After the surface water sampling survey did not indicate a major pollution problem the City shifted the emphasis of study from a detailed pollution study to a preliminary pollution study and a storm water drainage infrastructure assessment. The study included the surface water sampling survey; mail out questionnaire requesting information from the property owners on sewage disposal and storm drainage problems; and preparation of a preliminary storm drainage plan for the two study areas.

In order to provide a basis for evaluating the cost of future infrastructure improvements in the two study areas, preliminary layouts and preliminary cost estimates were prepared for possible sewage collection and storm drainage improvements in both study areas.

EXISTING DEVELOPMENT

Based on the Shasta County assessor's roll and the City's address map it is estimated that there are about 400 residences, several commercial businesses and one elementary school in the Summit City study area. The majority of the existing development is in the area south of Shasta Dam Blvd. and east of Churn Creek. The subdivision lot sizes vary from about 2,500 FT² in the central part of Summit City to about 1-acre in the Ranchera Pines Subdivision.

All of the existing development utilizes on-site sewage disposal systems. The on-site sewage disposal systems are regulated by the Shasta County Environmental Health Department even though the areas were incorporated as part of the City of Shasta Lake in July 1993.

PHASE I - INITIAL STUDY

REVIEW OF SOILS INFORMATION

The Soil Conservation Service (SCS) Soils Survey of Shasta County indicates basically six different soil types in the Summit City study area. The majority of the area consists of either the Auburn (AnB, AnD, AsD2, AtE2) or the Boomer (BkD, BkE) soil series. Auburn soils typically consist of about 5 inches of yellowish-red clay soil overlaying about 2 feet of yellowish-red gravelly clay loam. Weathered bedrock is generally at a depth of 15 to 32 inches. The SCS indicates severe limitations to septic systems because of shallow bedrock. Boomer soils typically consist of a surface layer of light brown gravelly loam about 3 inches thick overlaying about 11 inches of reddish-yellow gravelly sandy clay loam and about 30 inches of red gravelly clay loam, clay loam and silty clay loam. Red weathered and fractured green stone is normally encountered at depths of 40 inches to more than 60 inches. The SCS indicates severe limitation to septic systems due to moderately slow permeability.

The soils in the Riddle Road study area are primarily Auburn (AnB) soils as described above.

SURFACE WATER QUALITY SURVEY

In April and May of 1996, the City of Shasta Lake developed a stream sampling program that was conducted in conjunction with the Adopt-A-Watershed Program (Americorp) with the assistance of students from Central Valley High School. On April 4, April 18, May 18, and May 29, 1996, surface water samples were collected at the 22 monitoring locations shown on Figure 1 and 5 sites on Churn Creek at locations downstream of Ashby Road. The students tested the samples for temperature, pH, dissolved oxygen, conductivity, total dissolved solids,

and chlorides. In addition, a separate sample was sent to a commercial laboratory for total coliform, fecal coliform and ammonia concentration analyses. The testing results for total coliform and fecal coliform are shown in Table 1. As one can see the total coliform levels varied from a low 4 MPN/100 ml to 160,000 MPN/100 ml while the fecal coliform levels varied from a low of less than 2 MPN/100 ml to a high of 90,000 MPN/100 ml. While total coliform organisms are fairly widespread in the soil, the fecal coliform is an indicator organism for the potential presence of disease organisms. Water bodies with concentrations of 200 MPN/100 ml or greater of fecal coliform are considered unsafe for general human contact. While the high fecal coliform values obtained at sampling locations 8B, 8C and 11A may be indicative of failing septic systems in the area, it would be necessary to perform wet weather dye testing of the septic systems to confirm such failures.

The surface water testing results for ammonia and chlorides are shown in Table 2. While one might expect elevated concentrations of ammonia and chlorides in surface waters contaminated with septic tank effluent, the potential for dilution in the surface water body can cause a wide variation in the actual values. While a comparison of the fecal coliform test results in Table 1 with the ammonia and chloride test results in Table 2 does indicate a general correlation of higher ammonia and chloride concentrations when high fecal coliform concentrations were present, the results are not consistent. Therefore, no definite conclusions can be made without dye testing.

MAIL OUT QUESTIONNAIRE

In March 1997, the City mailed out a questionnaire to every property owner in the Summit City (578 total parcels) and Riddle Road (24 total parcels) study areas. The questionnaire requested information regarding their on-site sewage disposal system, storm drainage problems and household income levels. As of April 20, 1997, a total 230 of the questionnaires had been returned.

SUMMIT CITY AREA RESPONSE: Based upon a review of the current Shasta County assessor's roll, approximately 416 of the 578 parcels in the Summit City area have improvements on them, although some of the improvement values are very low. After discussing the situation with the County Assessor's Office, it is estimated that about 400 parcels are actually developed at this time. A total of 223 questionnaires were returned. An analysis of the returned questionnaires indicated a total of 182 residential dwellings, 3 commercial establishments, and 38 being either vacant or providing no response as to the status of the parcel. Therefore, it is estimated that approximately 185 of the developed parcels or about 46 percent responded by completing the questionnaire.

The following is a summary of the responses regarding the status of their on-site sewage disposal systems:

| <u>Category</u> | <u>Number of Premises</u> |
|---|---------------------------|
| Septic System Without Problems | 154 |
| Septic System with Problems (such as slow drainage, odors, liquid on surface or frequent pumping) | 13 |
| SUBTOTAL OF RESPONDING PREMISES | 167 |
| Approximate Total Number of Premises | 400 |
| Approximate Number of Premises Not Responding | 233 |

Of the 167 existing premises that responded regarding the status of their septic systems, about 92 percent indicated that they did not have any problems and about 8 percent indicated that they did have problems. The majority of the premises with septic system problems indicated that the problems occur primarily during the heavy rainfall periods.

In addition to the 13 premises that indicated they had septic system problems, a total of 21 premises indicated that their neighbors have septic system problems. When all of this information was plotted on a map of the area, it was found that a majority of the reported septic system problems were on the small lots east of Lake Blvd., between Rose Avenue on the

north and Oak Avenue on the south. This area is also a tributary to surface water sampling locations 8C and 11A which indicated the presence of relatively high fecal coliform concentrations.

With regard to the question related to existing storm water drainage problems, a total of 29 of the Summit City area property owners indicated problems with periodic flooding of their property and 150 indicated no problems. Thirteen of the parcels with storm water drainage problems were in the area of small lots that indicated the highest percentage of septic system problems.

RIDDLE ROAD AREA RESPONSE: Based upon a review of the current Shasta County assessor's roll, approximately 20 of the 24 parcels in the Riddle Road area have improvements on them, although some of the improvement values are very low. An analysis of the questionnaires received indicated a total of 5 residential dwellings, 1 commercial establishment, and 2 vacant parcels responded to the survey. Therefore, it is estimated that approximately 30 percent of the developed parcels responded by completing the questionnaire.

The following is a summary of the responses regarding the status of their on-site sewage disposal systems:

| <u>Category</u> | <u>Number of Premises</u> |
|---|---------------------------|
| Septic System Without Problems | 2 |
| Septic System with Problems (such as slow drainage, odors, liquid on surface or frequent pumping) | 4 |
| SUBTOTAL OF RESPONDING PREMISES | 6 |
| Approximate Total Number of Premises | 20 |
| Approximate Number of Premises Not Responding | 14 |

Of the 6 existing premises that responded regarding the status of their septic systems, one-third indicated that they did not have any problems and two-thirds indicated that they did have

problems. The majority of the premises with septic system problems indicated that the problems occur primarily during the heavy rainfall periods.

With regard to the question related to existing storm water drainage problems, one-third of the Riddle Road area property owners indicated problems with periodic flooding of their property and two-thirds indicated no problems.

PHASE II - PRELIMINARY STORM DRAINAGE STUDY

INTRODUCTION

The purpose of this preliminary storm drainage study is to evaluate the condition of the storm drain systems in the Summit City and Riddle Road areas. The scope of work includes calculating the flow rates at critical points along the major drainages, such as a pipe crossing a city street. Generally, only areas that were large enough to require a 24-inch or larger pipe were evaluated. However, additional review was made in the Summit City area west of Hill Blvd. and between Rose Avenue and Spruce Avenue. By identifying the areas with the biggest problems, and the greatest potential for property damage, drainage improvement projects can be prioritized and budgeted.

This preliminary drainage study does not determine the water surface profiles or the extent of flooding. Not all alternatives were evaluated. For example, piping the drainage from Rose Avenue to Spruce Avenue would be an effective but expensive alternative that could also be considered.

This report includes preliminary cost estimates to provide an approximate cost of the various improvements. These estimates can be used to establish priorities. Cost estimates based on a preliminary design should be completed prior to preparing grant applications.

DESCRIPTION

The Churn Creek watershed upstream from the Southern Pacific railroad track in Summit City is approximately 4.3 square miles. It begins in the hills adjacent to Lake Blvd. near the turn-off to the Centimudi boat launch at Shasta Lake. The creek runs along Lake Blvd. through

most of the study area before turning southeasterly under Hill Street and the railroad tracks at the southern end of the drainage area (see Figure 2). Two significant drainages, Rich Gulch and Little Churn Creek, enter Churn Creek from the west at the southern end of the study area. Approximately half of the runoff comes from these two creeks.

The soil survey of Shasta County classifies the soils in the area as predominately "gravelly clay loam" and "clay loam." These soils have moderate to moderately slow permeability with medium to rapid rates of runoff (see Figure 3).

Most of the area north of Shasta Dam Blvd. and west of Lake Blvd. is either owned by the federal government or is zoned for very low density development (parcels larger than 5 acres). Most of the area south of Shasta Dam Blvd. and east of Lake Blvd. is zoned for medium density residential development at a density ranging from 1 to 4 units per acre. There is also a small commercial area near the intersection of these two roads. (See Figures 4 and 5.) Most of the undeveloped areas in the watershed are covered with oaks, pines, and dense brush including chaparral, manzanita, buck brush, etc.

The Riddle Road area has one major drainage that crosses Indian Avenue and Arrowhead Avenue. The tributary area is approximately 113 acres. The area begins on the ridge north of Windsor Estates Subdivision and flows from north to south (see Figure 6). Both Riddle Road and Arrowhead Avenue are narrow gravel roads that cross the main drainage course.

The soils within this drainage have moderate permeability and medium to rapid rates of runoff.

The existing land use in most of this watershed is low density (2 to 5 acre parcels). The future land use plan is for medium density development of approximately 3 units/acre.

HYDROLOGY

Flow rates were calculated for the 25-year storm and the 100-year storm for two levels of development: (1) 1997 land use based on current development (See Figure 4) and (2) future land use based on current zoning (see Figure 5). The 25-year storm was used for sizing culverts for areas less than 160 acres. The 100-year storm was used for areas larger than 160 acres.

The Soil Conservation Service (SCS) hydrology method was used for calculating flow rates. Soil type, ground cover, and the land use are variables used to calculate a runoff coefficient called the Curve Number (CN). The time it takes for the water to travel through a drainage area is called the Time of Concentration (Tc). The Tc for overland flow is a function of the slope of the ground and the ground cover. The Tc for pipe or channel flow is a function of the slope and roughness of the pipe or channel. The maximum rainfall in 24 hours for a given return period (25 years or 100 years) is called P_{24} . Rainfall data from Shasta Dam was used to determine a P_{24} of 8.4 inches for the 25-year storm and 10.2 inches for the 100-year storm. Flowrates were calculated using these variables and a unit hydrograph based upon rainfall patterns typical for the area. These flows are summarized in Table 3 and Table 4.

In the Riddle Road area the P_{24} was also determined to be 8.4 inches for the 25-year storm and 10.2 inches for the 100-year storm using the isoplurial maps in the Shasta County Development Standards. The flows are summarized in Table 5.

HYDRAULICS AND COST ESTIMATES

Table 6 summarizes the existing facilities at the point of concentration (P.O.C.) for each subbasin and the recommended improvements. New culverts between two of these points should be sized based on the flow at the downstream P.O.C.

All culverts are sized based on the free flow of water through the culverts. The affect of backwater caused by inadequate culverts or partially blocked culverts was not analyzed.

Churn Creek flows through several culverts under private driveways north of Shasta Dam Blvd. Their adequacy was not evaluated. These culverts are privately owned and are maintained by the property owners.

The culverts at P.O.C. 1 and P.O.C. 2 are too small to carry the design flows. Additionally, the culverts at P.O.C. 2 (2-48" CMP's) are rusted out and need to be replaced. The rest of the City maintained culverts along Churn Creek are adequate to carry the 100-year storm. However, some of the reinforced concrete boxes are partially filled with sand and gravel. The City should establish a schedule to periodically inspect and maintain the storm drain system.

East of Hill Blvd. in Summit City, the storm drain system has more problems. There are two main drainages south of Rose Avenue that join together about 200 feet south of Oak Street on the east side of Lassen Street. The system typically has culvert pipes at the street and driveway crossings and an open ditch between the culverts. The system is difficult and expensive to maintain because it flows across private property as well as under and between buildings for much of its route. Even when the ditch is adjacent to the roadway, the presence of water throughout the year causes vegetation to grow rapidly, restricting the flow of water.

The inadequate existing culvert pipes between Rose Avenue and Ranchera Road (from P.O.C. 14 to 15 and P.O.C. 18 to 22) could be upgraded to the appropriate size for approximately \$260,000. However, if right-of-way needs to be acquired to enlarge the existing pipes and ditches, the cost could be considerably higher.

An alternative to replacing most of the pipes between Rose Avenue and Ranchera Road is to construct a bypass on the north side of Rose Avenue. Beginning at P.O.C. 18, a new pipe or a combination of pipes and an open channel could be constructed from P.O.C. 18 to Churn Creek on the west side of Lake Blvd. Most of the property is presently owned by the City of

Shasta Lake. The approximate cost for this bypass is \$245,000. This would divert about 125 cfs that would normally flow through the residential area during a 25-year storm. The Churn Creek drainage structures downstream of this diversion are capable of carrying the additional flows.

The bypass would make the existing pipes south of Rose Avenue adequate in size but does not take into account replacement of the pipes that are rusted out. This problem could probably be solved cost effectively by installing a smaller plastic pipe, such as high density polyethylene (HDPE) pipe, into the existing pipe and grouting the void.

The ditches would continue to be a maintenance expense, but with less water flowing through them, they should flood less frequently.

A third alternative would be to install curbs, gutters, catch basins and a complete underground storm drain system. The cost of this alternative was not determined, but it would be substantially more expensive.

To help identify and verify drainage problem areas, a questionnaire was mailed to the residents in the study area under the initial phase of the study. Many of the drainage problems identified in the mail-out questionnaire were localized problems not directly related to the storm drain system. For example, the fire station has problems with high groundwater coming up through the floors of some of the rooms. This problem is probably not caused by an inadequate area wide drainage system, but appears to be caused by inadequate local drainage around the building. The ground slopes towards the building in some areas rather than away from the building. If groundwater is the source of flooding, a french drain could be designed to pipe the groundwater to the ditch east of the fire station.

There were a few complaints about flooding near Beacon Street and Pine Avenue. Although the drainage area is small, the 12-inch pipes are plugged with sediment forcing the water to

flood across the streets. These small pipes plug easily and require regular maintenance. They should be replaced with 18-inch pipes with concrete inlet structures.

The two 48-inch culverts across Indian Avenue (P.O.C. 1) and the small bridge across Arrowhead Avenue (P.O.C. 2) in the Riddle Road area are adequate hydraulically; but, they are not long enough to provide a standard roadway width and the existing culverts are not installed low enough to properly channel the flow. Both roads are narrow gravel roads. When flows exceed the capacity of the culverts or bridge, it appears that the water would flow over the road without damaging nearby houses. Erosion damage to the roadway is repaired when the high water subsides. If and when the roads are constructed to City standards and accepted by the City for maintenance, both crossings should be replaced with two new 48-inch culverts.

CONCEPTUAL SEWAGE COLLECTION SYSTEMS

SUMMIT CITY AREA

A preliminary layout for a conventional gravity sewage collection system to serve most of the Summit City study area is shown on Figure 7. The 15-inch interceptor sewer and main pump station to the existing City wastewater treatment plant would be sized to handle the entire flow from the potential sewered area boundary which is slightly larger than the study area.

Collection sewers are also shown to serve most of the existing parcels in the study area.

The total estimated capital project cost for this conceptual sewer system layout to serve most of the Summit City Area is \$6,470,000 (excluding financing cost) as shown in Table 7. If one assumes that the cost of this collection system is uniformly spread over 500 household equivalents, then the cost per household equivalent would be about \$13,000. In addition, each household equivalent would need to pay the City's sewer connection fee which is currently about \$2,800. Thus, the total capital cost per household equivalent would be about \$15,800. Assuming that sufficient documentation can be obtained in the future for the project to qualify for a 75 percent grant the capital cost per household could be reduced to about \$4,000. Based upon a Rural Development 5 percent loan for 40 years, the annual debt service on \$4,000 would be about \$232 per year or about \$19.30 per month. When the monthly debt service is added to the City's existing monthly service charge for operation and maintenance the total monthly cost would be about \$36.60 per month in 1997 dollars. Without any grant funding, the total monthly cost would be about \$95.00 per month.

As an alternative to sewerage the entire area, initially the sewage collection system could be configured as shown on Figure 8 to serve the higher density small parcel size areas. The 15-inch interceptor would still allow for ultimately sewerage the entire study area if necessary. The total estimated capital project cost for this conceptual sewer system layout to serve the high density portion of the Summit City Area is \$4,436,000 as shown in Table 8. If one

assumes that the cost of this initial collection system is uniformly spread over the about 350 household equivalents that it would serve, then the cost per household equivalent would be about the same as the cost indicated for the sewer system that would serve most of the service area. If additional areas are connected in the future then they could be required to contribute for their share of the main line sewers that they would tie into.

RIDDLE ROAD AREA

A preliminary layout for a conventional gravity sewage collection system to serve the Riddle Road study area is shown on Figure 9.

The total estimated capital project cost for this conceptual sewer system layout to serve the Riddle Road Area is \$466,000 (excluding financing cost) as shown in Table 9. If one assumes that the cost of this collection system is uniformly spread over 24 household equivalents, then the cost per household equivalent would be about \$19,400. In addition, each household equivalent would need to pay the City's connection fee which is currently about \$2,800. Thus, the total capital cost per household equivalent would be about \$22,200. Assuming that sufficient documentation can be obtained in the future for the project to qualify for a 75 percent grant the capital cost per household could be reduced to about \$5,550. Based upon a Rural Development 5 percent loan for 40 years, the annual debt service on \$5,500 would be about \$322 per year or about \$27.00 per month. When the monthly debt service is added to the City's existing monthly service charge for operation and maintenance the total monthly cost would be about \$44.30 per month in 1997 dollars. Without any grant funding, the total monthly cost would be about \$125.00 per month.

TABLES

TABLE 1

SUMMIT CITY AREA SURFACE WATER QUALITY MONITORING DATA
TOTAL COLIFORM AND FECAL COLIFORM TEST RESULTS

| TOTAL COLIFORM (MPN/100ml) | | | | | FECAL COLIFORM (MPN/100ml) | | | |
|----------------------------|--------|---------|---------|---------|----------------------------|---------|---------|---------|
| Station Number | 4/4/96 | 4/18/96 | 5/18/96 | 5/29/96 | 4/4/96 | 4/18/96 | 5/18/96 | 5/29/96 |
| R-1 | 900 | 900 | 93 | > 1600 | 23 | 50 | 43 | 130 |
| R-2 | 90 | 500 | 240 | > 1600 | 30 | 50 | 43 | 220 |
| R-3 | 280 | 500 | > 2400 | 1600 | 70 | 130 | 93 | 70 |
| R-4 | 170 | 500 | 460 | > 1600 | 80 | 130 | 93 | 220 |
| R-5 | 110 | 500 | > 2400 | 900 | 50 | 80 | 150 | 30 |
| 6A | 300 | 900 | 1100 | 280 | 27 | 70 | 23 | 23 |
| 6B | > 1600 | 500 | 1100 | > 1600 | > 1600 | 500 | 460 | > 1600 |
| 7 | 70 | 1600 | 460 | 300 | 23 | 50 | 9 | 8 |
| 7A | 500 | 900 | 1100 | 1600 | 17 | 130 | 9 | 23 |
| 7B | 4 | 300 | 23 | 900 | > 2 | 13 | 9 | 23 |
| 7C | 21 | 280 | N/A | 900 | 4 | 280 | N/A | 240 |
| 8 | 900 | 900 | 75 | > 1600 | 40 | 30 | 23 | 60 |
| 8A | 500 | > 1600 | 150 | 500 | 80 | 500 | 43 | 130 |
| 8B | 900 | > 1600 | > 2400 | 3000 | 900 | 1600 | 120 | 500 |
| 8C | 220 | 900 | > 2400 | 1600 | 80 | 300 | > 2400 | 50 |
| 9 | 500 | 900 | 460 | 300 | 13 | 60 | 4 | 240 |
| 9A | 170 | 300 | 1100 | 170 | 11 | 50 | 15 | 4 |
| 9B | 300 | 12 | 43 | 120 | 13 | < 2 | < 3 | 2 |
| 10 | 500 | 240 | 460 | 900 | 50 | 50 | 9 | 90 |

TABLE 1 (Continued)

SUMMIT CITY AREA SURFACE WATER QUALITY MONITORING DATA
 TOTAL COLIFORM AND FECAL COLIFORM TEST RESULTS

| TOTAL COLIFORM (MPN/100ml) | | | | | FECAL COLIFORM (MPN/100ml) | | | |
|----------------------------|-------|-------|-------|--------|----------------------------|-------|-------|--------|
| 10A | 17 | 30 | >2400 | 220 | <2 | <2 | 15 | 7 |
| 11 | >1600 | 300 | >2400 | 500 | 500 | 30 | 7 | 70 |
| 11A | >1600 | >1600 | >2400 | 160000 | >1600 | >1600 | >2400 | 90,000 |
| 11B | 50 | 170 | >2400 | 1600 | 2 | 8 | 233 | 70 |
| 11C | 23 | 300 | 1100 | 280 | 4 | 50 | 9 | 110 |
| 12 | 500 | 300 | 1100 | 140 | 13 | 13 | 23 | 13 |
| 12A | 900 | 500 | >2400 | N/A | 130 | 80 | 240 | N/A |
| 13 | 4 | 8 | 13 | 50 | <2 | <2 | <3 | <2 |

TABLE 2

SUMMIT CITY AREA SURFACE WATER QUALITY MONITORING DATA
AMMONIA AND CHLORIDE TEST RESULTS

| Station Number | AMMONIA (mg/L or N) | | | CHLORIDES (mg/L) | | | |
|----------------|---------------------|---------|---------|------------------|---------|---------|---------|
| | 4/4/96 | 4/18/96 | 5/29/96 | 4/4/96 | 4/18/96 | 5/18/96 | 5/24/96 |
| R-1 | n | n | 0.35 | 3.5 | 3.8 | 4.0 | 3.7 |
| R-2 | n | n | 0.12 | 8.6 | 6.1 | 3.8 | 2.2 |
| R-3 | n | n | 0.11 | 7.8 | 6.5 | 3.6 | 2.9 |
| R-4 | n | 0.11 | n | 7.5 | 6.7 | 4.1 | 2.7 |
| R-5 | n | n | 0.36 | 7.7 | 7.2 | 5.9 | 5.6 |
| 6A | n | n | 0.22 | 3.3 | 2.7 | 2.7 | 2.9 |
| 6B | n | n | 0.23 | 1.1 | 0.6 | 1.3 | 1.1 |
| 7 | n | 0.12 | n | 3.3 | --- | 3.3 | 1.4 |
| 7A | n | n | 0.12 | 3.0 | 1.8 | 2.7 | 1.6 |
| 7B | n | n | n | 1.3 | 0.7 | 1.7 | 1.2 |
| 7C | n | n | 0.33 | 0.7 | 1.4 | dry | 0.2 |
| 8 | n | n | 0.46 | 2.9 | 2.8 | 2.9 | 1.5 |
| 8A | n | n | 0.18 | 4.7 | 7.2 | 4.7 | 4.6 |
| 8B | n | n | 0.81 | 4.6 | 6.8 | 1.6 | 4.5 |
| 8C | n | n | 0.33 | 3.1 | 2.2 | 2.3 | --- |
| 9 | n | n | 0.23 | 1.5 | 1.6 | 1.5 | 0.8 |
| 9A | n | n | 0.12 | 0.9 | 1.0 | 1.0 | 0.3 |
| 9B | n | n | 0.31 | 0.8 | 0.6 | 0.8 | 0.4 |
| 10 | n | n | n | 1.7 | 1.4 | 2.0 | 0.9 |
| 10A | n | n | 0.06 | 0.8 | --- | 0.9 | 0.1 |
| 11 | n | n | 0.19 | --- | 1.8 | 1.9 | 1.2 |
| 11A | n | 0.49 | 0.11 | 2.5 | 2.6 | 3.8 | 0.7 |
| 11B | n | n | 0.12 | 0.9 | 0.7 | 1.8 | 0.3 |
| 11C | n | n | 0.07 | 2.7 | 2.3 | 2.2 | 3.3 |
| 12 | n | n | 0.13 | 1.4 | 2.7 | 1.5 | --- |
| 12A | n | 0.09 | --- | 1.8 | 3.5 | 2.2 | --- |
| 13 | n | 0.35 | 0.13 | 0.6 | 0.5 | 0.9 | 0.5 |

n = nondetectable @ 0.05 mg/L detection limit

**TABLE 3
CALCULATED FLOWS
FOR SUMMIT CITY
1997 LAND USE**

| Subbasin Number | Area in Acres | | Time of Concentration Tc (Min.) | Curve Number CN | Peak Flows in cfs | |
|-----------------|---------------|------------|---------------------------------|-----------------|-------------------|----------|
| | Subbasin | Cumulative | | | 25-Year | 100-Year |
| 1 | 109 | | 44 | 66 | 84 | 118 |
| 2 | 380 | | 51 | 74 | 364 | 486 |
| Combine | | 489 | 51 | | 447 | 605 |
| 3 | 125 | | 50 | 78 | 141 | 184 |
| 4 | 163 | | 59 | 76 | 153 | 203 |
| Combine | | 777 | 59 | | 736 | 987 |
| 5 | 90 | | 64 | 69 | 68 | 94 |
| Combine | | 867 | 64 | | 802 | 1079 |
| 6 | 29 | | 69 | 80 | 29 | 38 |
| Combine | | 896 | 69 | | 830 | 1114 |
| 7 | 18 | | 24 | 78 | 24 | 31 |
| 8 | 16 | | 28 | 80 | 22 | 29 |
| Combine | | 34 | 28 | | 46 | 60 |
| 9 | 9 | | 71 | 80 | 9 | 12 |
| Combine | | 939 | 71 | | 876 | 1174 |
| 10 | 511 | | 67 | 67 | 362 | 506 |
| 11 | 930 | | 81 | 71 | 697 | 952 |
| 12 | 32 | | 81 | 80 | 31 | 40 |
| Combine | | 2412 | 81 | | 1918 | 2607 |
| 13 | 7 | | 22 | 78 | 9 | 12 |
| 14 | 17 | | 26 | 77 | 22 | 29 |
| Combine | | 24 | 26 | | 31 | 41 |
| 15 | 17 | | 30 | 84 | 26 | 33 |
| Combine | | 41 | 30 | | 57 | 74 |
| 16 | 26 | | 31 | 78 | 34 | 45 |
| 17 | 13 | | 20 | 66 | 12 | 17 |
| 18 | 37 | | 34 | 62 | 25 | 37 |
| Combine | | 76 | 34 | | 71 | 97 |
| 19 | 32 | | 39 | 74 | 34 | 46 |
| Combine | | 108 | 39 | | 105 | 143 |
| 20 | 27 | | 43 | 70 | 24 | 33 |
| Combine | | 176 | 43 | | 184 | 247 |
| 21 | 10 | | 45 | 68 | 8 | 12 |
| Combine | | 186 | 45 | | 192 | 258 |
| 22 | 39 | | 51 | 68 | 31 | 43 |
| Combine | | 225 | 51 | | 220 | 298 |
| 23 | 85 | | 91 | 62 | 43 | 63 |
| Combine | | 2722 | 91 | | 2133 | 2908 |

**TABLE 4
CALCULATED FLOWS
FOR SUMMIT CITY
FUTURE LAND USE**

| Subbasin Number | Area in Acres | | Time of Concentration Tc (Min.) | Curve Number CN | Peak Flows in cfs | |
|-----------------|---------------|------------|---------------------------------|-----------------|-------------------|----------|
| | Subbasin | Cumulative | | | 25-Year | 100-Year |
| 1 | 109 | | 44 | 66 | 84 | 118 |
| 2 | 380 | | 51 | 74 | 364 | 486 |
| Combine | | 489 | 51 | | 447 | 605 |
| 3 | 125 | | 50 | 79 | 144 | 188 |
| 4 | 163 | | 59 | 77 | 158 | 207 |
| Combine | | 777 | 59 | | 744 | 995 |
| 5 | 90 | | 64 | 70 | 71 | 97 |
| Combine | | 867 | 64 | | 812 | 1089 |
| 6 | 29 | | 69 | 85 | 32 | 41 |
| Combine | | 896 | 69 | | 843 | 1128 |
| 7 | 18 | | 24 | 84 | 27 | 35 |
| 8 | 16 | | 28 | 85 | 25 | 31 |
| Combine | | 34 | 28 | | 52 | 66 |
| 9 | 9 | | 71 | 84 | 10 | 13 |
| Combine | | 939 | 71 | | 895 | 1193 |
| 10 | 511 | | 67 | 68 | 375 | 521 |
| 11 | 930 | | 81 | 71 | 697 | 952 |
| 12 | 32 | | 81 | 81 | 32 | 41 |
| Combine | | 2412 | 81 | | 1948 | 2640 |
| 13 | 7 | | 22 | 81 | 10 | 13 |
| 14 | 17 | | 26 | 84 | 26 | 33 |
| Combine | | 24 | 26 | | 36 | 46 |
| 15 | 17 | | 30 | 84 | 26 | 33 |
| Combine | | 41 | 30 | | 62 | 76 |
| 16 | 26 | | 31 | 79 | 35 | 46 |
| 17 | 13 | | 20 | 75 | 17 | 21 |
| 18 | 37 | | 34 | 72 | 37 | 50 |
| Combine | | 76 | 34 | | 89 | 116 |
| 19 | 32 | | 39 | 78 | 38 | 50 |
| Combine | | 108 | 39 | | 126 | 165 |
| 20 | 27 | | 43 | 75 | 28 | 37 |
| Combine | | 176 | 43 | | 214 | 278 |
| 21 | 10 | | 45 | 68 | 8 | 12 |
| Combine | | 186 | 45 | | 222 | 289 |
| 22 | 39 | | 51 | 68 | 31 | 43 |
| Combine | | 225 | 51 | | 250 | 329 |
| 23 | 85 | | 91 | 64 | 47 | 67 |
| Combine | | 2722 | 91 | | 2191 | 2970 |

SHASTA DAM BLVD

TABLE 5
CALCULATED FLOWS
FOR RIDDLE ROAD AREA
1997 AND FUTURE LAND USE

| Sub-basin Name | Area Acres | 1997 Land Use Peak Flows in CFS | | Future Land Use Peak Flows in CFS | |
|------------------|---------------|------------------------------------|----------|--------------------------------------|----------|
| | | 25 Year | 100-Year | 25-Year | 100-Year |
| Indian Avenue | 91 | 108 | 141 | 144 | 181 |
| Arrowhead Avenue | 113 | 127 | 167 | 178 | 225 |

**TABLE 6
FOR SUMMIT CITY AREA
FACILITY EVALUATION**

| P.O.C No. | Cross Street | Existing Facility | Comments | Approx. Capacity | Level of Protection | Design Flow (CFS) | Recommended Improvements | Approx. Cost |
|-----------|---------------------------|---------------------------------|------------------------------|------------------|---------------------|-------------------|--------------------------|--------------|
| 1 | Lake Blvd | 30" CMP | | 35 | 25 | 84 | Add a 42" RCP | \$9,000 |
| 2 | Lake Blvd | 2-48" CMP | Bottom rusted out | 230 | 100 | 605 | 8'x4' RCB | \$16,500 |
| 3 | Lake Blvd | 10", 15", 24" 48" CMP | | 115 | 25 | 144 | Add a 24" RCP | \$6,000 |
| 4 | Duval Drive | Double 10'x6' RCB | Partially filled with gravel | 1150 | 100 | 995 | None needed | |
| 5 | Shasta Dam Blvd | 12' CMP | | 1800 | 100 | 1089 | None needed | |
| 6 | Buckeye Street | Triple 10'x5' RCB | | 2700 | 100 | 1128 | None needed | |
| 7 | Shasta Dam Blvd | 36" CMP & 12" CMP | | 60 | 25 | 27 | None needed | |
| 8 | Hill Blvd | 42"x29" CMPA | | 55 | 25 | 52 | None needed | |
| 9 | Lake Blvd | Double 8'x6' RCB | | 1700 | 100 | 1193 | None needed | |
| 10 | Lake Blvd | Triple 8'x4' RCB | Partially filled with gravel | 1500 | 100 | 521 | None needed | |
| 11 | Lake Blvd | Double 8'x6' RCB | Partially filled with gravel | 1700 | 100 | 952 | None needed | |
| 12 | Hill Blvd | Double 12'x8' RCB | | 2900 | 100 | 2640 | None needed | |
| 13 | Shasta Dam Blvd | 12" CMP | | 5 | 25 | 10 | 18" RCP | \$5,000 |
| 14 to 15 | Rose Ave.-Lassen St. | 24" CMP's | | 18 | 25 | 36 | Replace w/36" RCP's | \$92,000 |
| 15 | Lassen Street | 42"x29" CMPA | | 65 | 25 | 62 | None needed | |
| 16 | Shasta Dam Blvd | 2 - 18" CMP | | 24 | 25 | 35 | Add a 18" RCP | \$5,000 |
| 17 | Shasta Dam Blvd | 30" CMP | | 35 | 25 | 17 | None needed | |
| 18 to 19 | Rose Ave.-Beacon St. | 36" CMP's | | 55 | 25 | 89 | Replace w/48" RCP's | \$83,000 |
| 19 to 20 | Beacon Street-Spruce Ave. | 48" CMP & 30" CMP | Bottom rusted out | 160 | 25 | 126 | Replace existing pipes | \$25,000 |
| 20 | Spruce Avenue | 2-48" CMP | Bottom rusted out | 230 | 100 | 278 | Replace w/2-54" RCP | \$30,000 |
| 21 | Ranchera Road | 2-64"x43" CMPA | | 300 | 100 | 289 | None needed | |
| 22 | Ranchera Road | 1-48" CMP | | 115 | 100 | 329 | Replace w/2-54" RCP | \$30,000 |
| 23 | SP Railroad | 15'x15' concrete parabolic arch | | 3000± | 100 | 2970 | None needed | |

TABLE 7
 PRELIMINARY COST ESTIMATE
 SEWAGE COLLECTION SYSTEM TO SERVE
 MOST OF THE SUMMIT CITY AREA

| NO. | DESCRIPTION | QUANTITY | UNITS | UNIT PRICE | ENR 5250 TOTAL COST |
|---|--|----------|-------|--------------|------------------------|
| 1 | 6 - inch sewer | 33000 | l.f. | \$50.00 | \$1,650,000.00 |
| 2 | 8 - inch sewer | 5700 | l.f. | \$55.00 | \$313,500.00 |
| 3 | 10 - inch sewer | 2300 | l.f. | \$70.00 | \$161,000.00 |
| 4 | 12 - inch sewer | 5300 | l.f. | \$75.00 | \$397,500.00 |
| 5 | 15 - inch sewer | 5600 | l.f. | \$80.00 | \$448,000.00 |
| 6 | Bore & Jack Railroad | 60 | l.f. | \$450.00 | \$27,000.00 |
| 7 | Manholes | 120 | each | \$2,200.00 | \$264,000.00 |
| 8 | Rod holes | 20 | each | \$560.00 | \$11,200.00 |
| 9 | 4 - inch lateral | 17000 | l.f. | \$25.00 | \$425,000.00 |
| 10 | Pump Station No. 1 (Without Standby Power) | 1 | l.s. | \$75,000.00 | \$75,000.00 |
| 11 | Pump Station No. 2 (Without Standby Power) | 1 | l.s. | \$75,000.00 | \$75,000.00 |
| 11 | Pump Station No. 3 (Without Standby Power) | 1 | l.s. | \$75,000.00 | \$75,000.00 |
| 12 | Main Pump Station (With Standby Power) | 1 | l.s. | \$470,000.00 | \$470,000.00 |
| 13 | 4 - inch force main | 1600 | l.f. | \$25.00 | \$40,000.00 |
| 14 | 12 - inch force main | 1400 | l.f. | \$40.00 | \$56,000.00 |
| 15 | Easement and Land Acquisitions | 1 | l.s. | \$200,000.00 | \$200,000.00 |
| TOTAL ESTIMATED CONSTRUCTION COSTS | | | | | \$4,688,200.00 |
| Contingency Allowance (15%) | | | | | \$703,200.00 |
| TOTAL ESTIMATED IMPROVEMENT COST INCLUDING CONTINGENCY ALLOWANCE | | | | | \$5,391,400.00 |
| Indirect Costs and Engineering (20%) | | | | | \$1,078,300.00 |
| TOTAL ESTIMATED CAPITAL PROJECT COST MAY 1997 DOLLARS (excluding financing cost) | | | | | \$6,469,700.00 |

TABLE 8
 PRELIMINARY COST ESTIMATE
 SEWAGE COLLECTION SYSTEM TO SERVE
 THE HIGH DENSITY PORTION OF SUMMIT CITY AREA

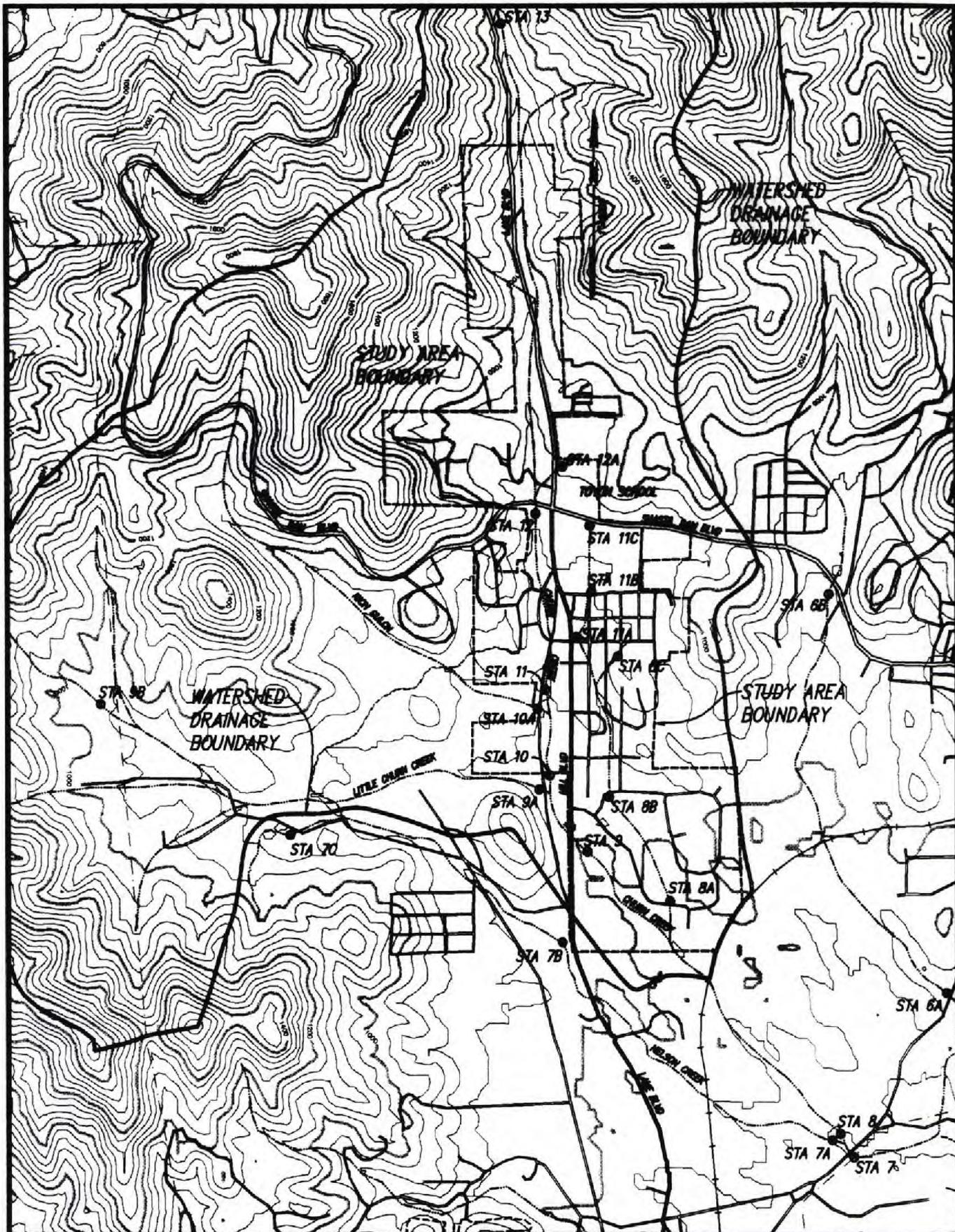
| NO. | DESCRIPTION | QUANTITY | UNITS | UNIT PRICE | ENR 5250 TOTAL COST |
|---|--|----------|-------|--------------|------------------------|
| 1 | 6 - inch sewer | 11700 | l.f. | \$50.00 | \$585,000.00 |
| 2 | 8 - inch sewer | 3100 | l.f. | \$55.00 | \$170,500.00 |
| 3 | 10 - inch sewer | 2300 | l.f. | \$70.00 | \$161,000.00 |
| 4 | 12 - inch sewer | 4900 | l.f. | \$75.00 | \$367,500.00 |
| 5 | 15 - inch sewer | 5600 | l.f. | \$80.00 | \$448,000.00 |
| 6 | Bore & Jack Railroad | 60 | l.f. | \$450.00 | \$27,000.00 |
| 7 | Manholes | 86 | each | \$2,200.00 | \$189,200.00 |
| 8 | Rod holes | 16 | each | \$560.00 | \$9,000.00 |
| 9 | 4 - inch lateral | 12700 | l.f. | \$25.00 | \$317,500.00 |
| 10 | Pump Station No. 1 (Without Standby Power) | 1 | l.s. | \$75,000.00 | \$75,000.00 |
| 11 | Pump Station No. 2 (With Standby Power) | 1 | l.s. | \$175,000.00 | \$175,000.00 |
| 11 | Pump Station No. 3 | 0 | l.s. | \$0.00 | \$0.00 |
| 12 | Main Pump Station (With Standby Power) | 1 | l.s. | \$400,000.00 | \$400,000.00 |
| 13 | 4 - inch force main | 600 | l.f. | \$25.00 | \$15,000.00 |
| 14 | 8 - inch force main | 1300 | l.f. | \$30.00 | \$39,000.00 |
| 15 | 12 - inch force main | 1400 | l.f. | \$40.00 | \$56,000.00 |
| 16 | Easement and Land Acquisitions | 1 | l.s. | \$180,000.00 | \$180,000.00 |
| TOTAL ESTIMATED CONSTRUCTION COSTS | | | | | \$3,214,700.00 |
| Contingency Allowance (15%) | | | | | \$482,200.00 |
| TOTAL ESTIMATED IMPROVEMENT COST INCLUDING CONTINGENCY ALLOWANCE | | | | | \$3,696,900.00 |
| Indirect Costs and Engineering (20%) | | | | | \$739,400.00 |
| TOTAL ESTIMATED CAPITAL PROJECT COST MAY 1997 DOLLARS (excluding financing cost) | | | | | \$4,436,300.00 |

File: TABLE-9
 Date rev.5/28/97

TABLE 9
 PRELIMINARY COST ESTIMATE
 SEWAGE COLLECTION SYSTEM TO SERVE
 THE RIDDLE ROAD AREA

| NO. | DESCRIPTION | QUANTITY | UNITS | UNIT PRICE | ENR 5250 TOTAL COST |
|---|--------------------------------------|----------|-------|-------------|------------------------|
| 1 | 6 - inch sewer (class A1 trench) | 800 | l.f. | \$50.00 | \$40,000.00 |
| 2 | 6 - inch sewer (class A4 trench) | 2850 | l.f. | \$35.00 | \$99,800.00 |
| 3 | 6 - inch sewer (class C trench) | 2100 | l.f. | \$20.00 | \$42,000.00 |
| 4 | Manholes | 18 | each | \$2,000.00 | \$36,000.00 |
| 5 | Rod holes | 1 | each | \$560.00 | \$600.00 |
| 6 | 4 - inch lateral | 250 | l.f. | \$15.00 | \$3,800.00 |
| 7 | Individual Pump System | 1 | l.s. | \$5,000.00 | \$5,000.00 |
| 8 | 1.5 - inch force main | 300 | l.f. | \$10.00 | \$3,000.00 |
| 9 | Pump Station (Without Standby Power) | 1 | l.s. | \$75,000.00 | \$75,000.00 |
| 10 | 4 - inch force main | 800 | l.f. | \$25.00 | \$20,000.00 |
| 11 | Easement and Land Acquisitions | 1 | l.s. | \$28,000.00 | \$28,000.00 |
| TOTAL ESTIMATED CONSTRUCTION COSTS | | | | | \$353,200.00 |
| Contingency Allowance (10%) | | | | | \$35,300.00 |
| TOTAL ESTIMATED IMPROVEMENT COST INCLUDING CONTINGENCY ALLOWANCE | | | | | \$388,500.00 |
| Indirect Costs and Engineering (20%) | | | | | \$77,700.00 |
| TOTAL ESTIMATED CAPITAL PROJECT COST MAY 1997 DOLLARS (excluding financing cost) | | | | | \$466,200.00 |

FIGURES



DATE
5/97



WATER QUALITY SAMPLING
STATION LOCATIONS

FIGURE 1
JOB # 110.24

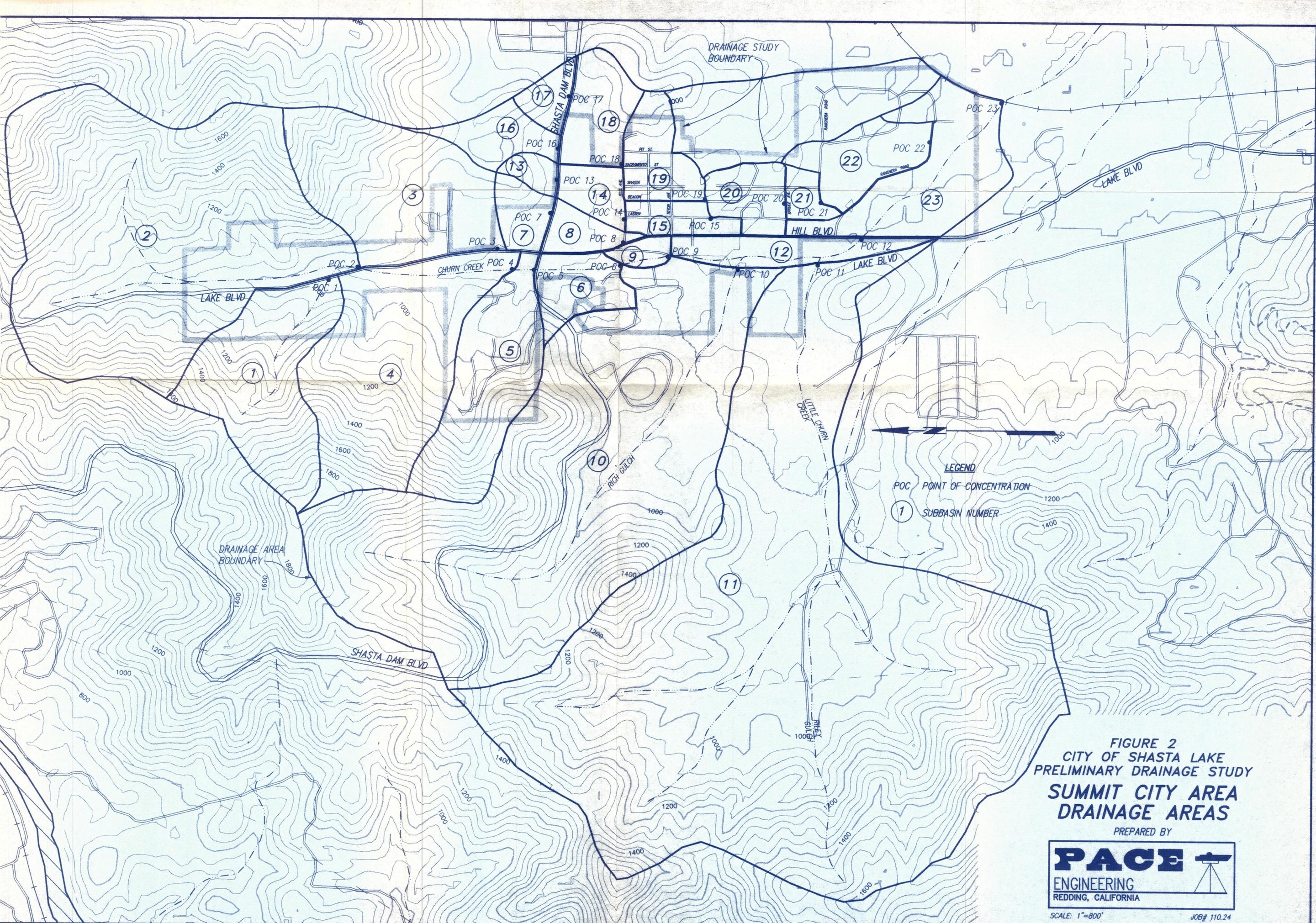
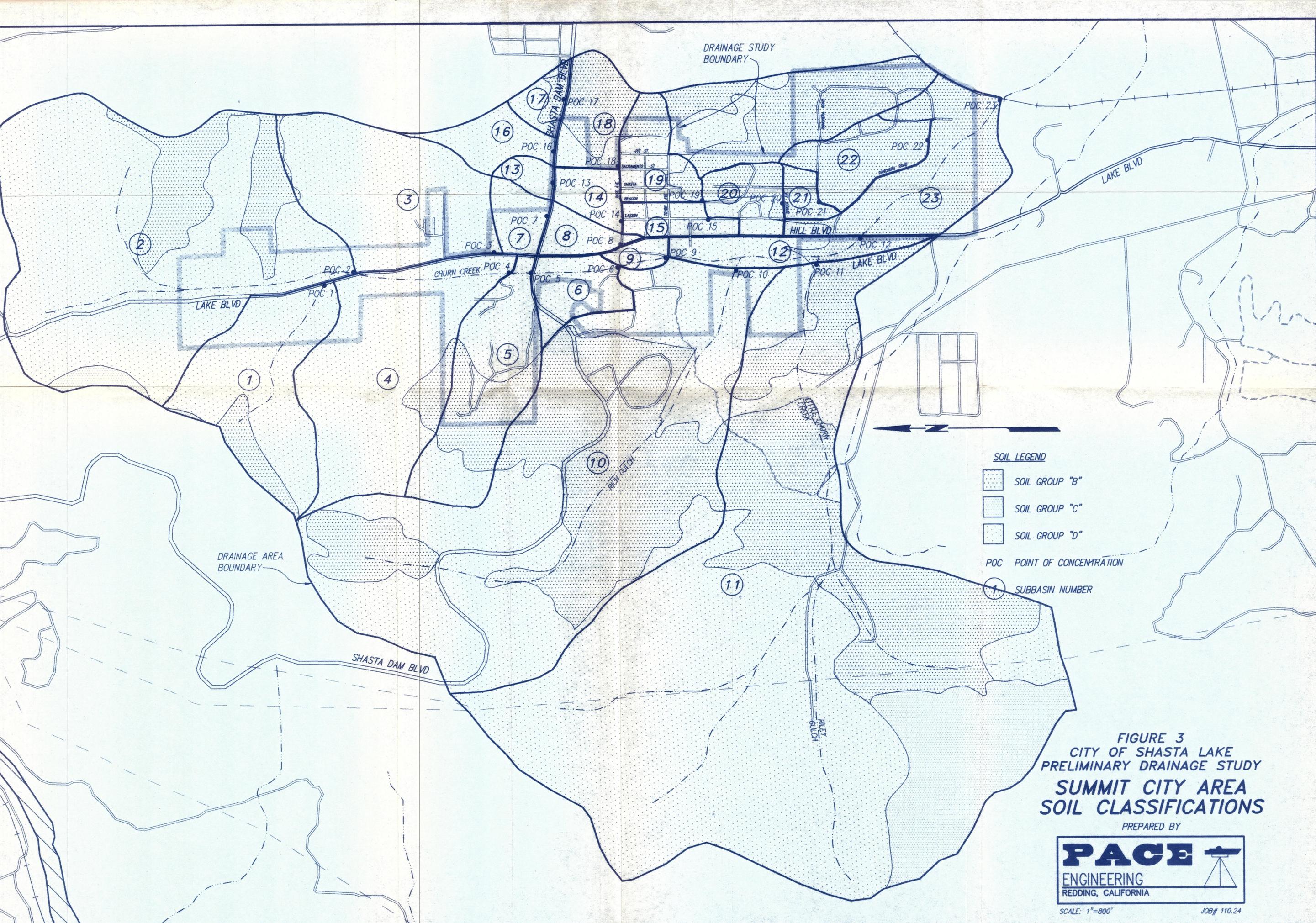


FIGURE 2
 CITY OF SHASTA LAKE
 PRELIMINARY DRAINAGE STUDY
**SUMMIT CITY AREA
 DRAINAGE AREAS**
 PREPARED BY

PACE 
 ENGINEERING
 REDDING, CALIFORNIA

SCALE: 1"=800'

JOB# 110.24



- SOIL LEGEND**
-  SOIL GROUP "B"
 -  SOIL GROUP "C"
 -  SOIL GROUP "D"
 -  POC POINT OF CONCENTRATION
 -  SUBBASIN NUMBER

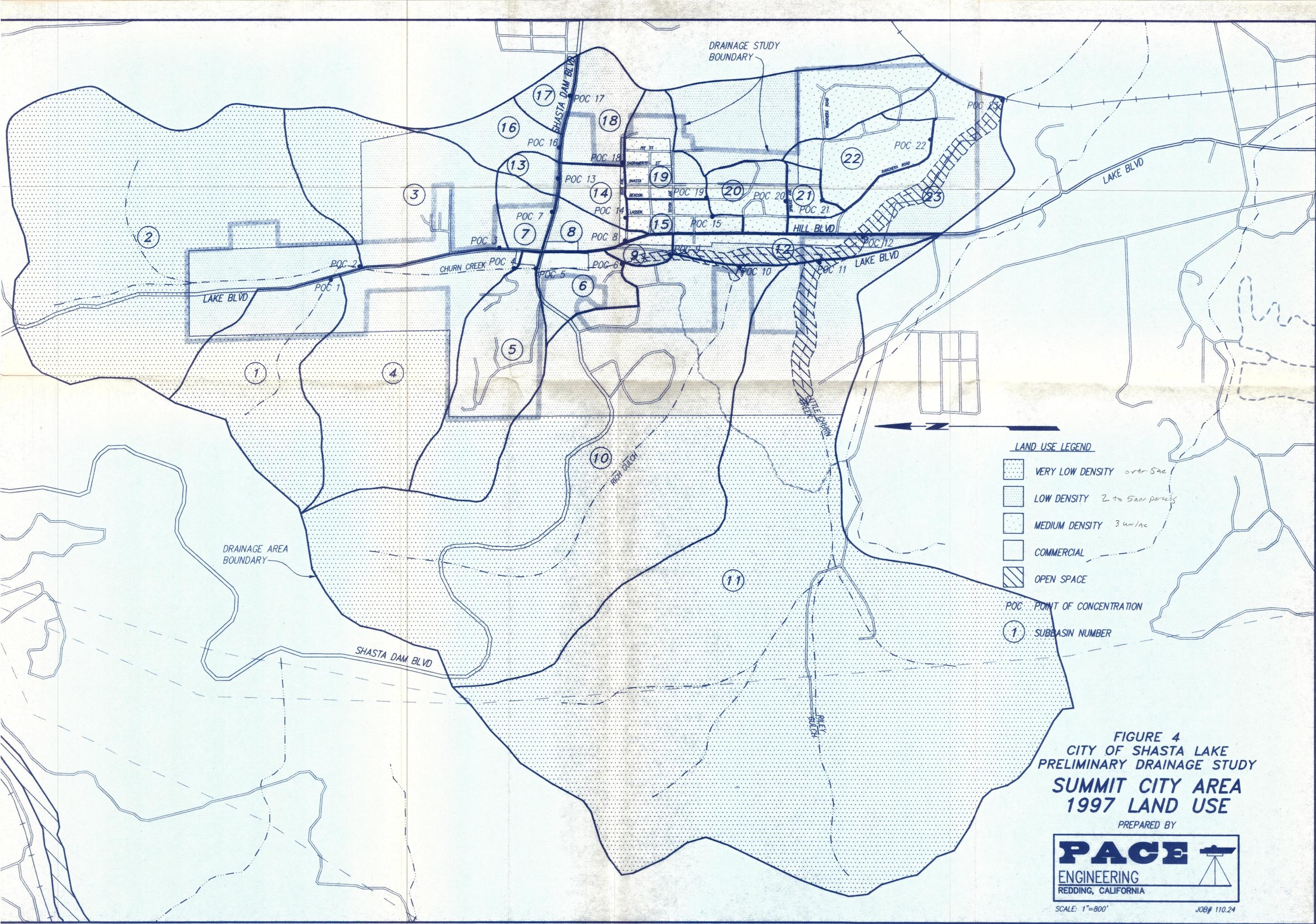
FIGURE 3
CITY OF SHASTA LAKE
PRELIMINARY DRAINAGE STUDY
SUMMIT CITY AREA
SOIL CLASSIFICATIONS

PREPARED BY



SCALE: 1"=800'

JOB# 110.24



- LAND USE LEGEND**
-  VERY LOW DENSITY *over 5ac*
 -  LOW DENSITY *2 to 5ac parcels*
 -  MEDIUM DENSITY *3 ac/acre*
 -  COMMERCIAL
 -  OPEN SPACE
 - POC: POINT OF CONCENTRATION
 -  SUBBASIN NUMBER

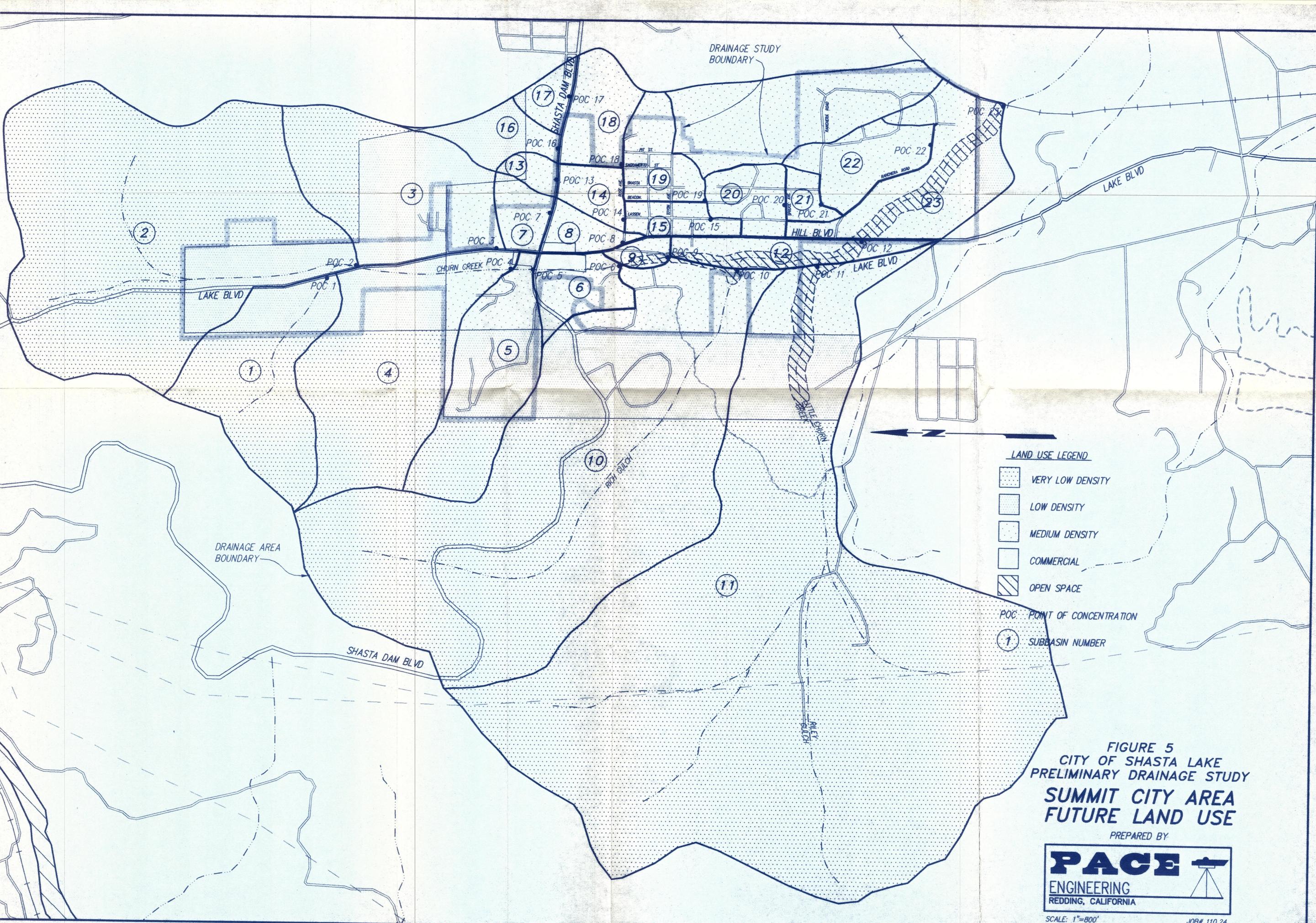
FIGURE 4
CITY OF SHASTA LAKE
PRELIMINARY DRAINAGE STUDY
SUMMIT CITY AREA
1997 LAND USE

PREPARED BY



SCALE: 1"=800'

JOB# 110.24



LAND USE LEGEND

-  VERY LOW DENSITY
-  LOW DENSITY
-  MEDIUM DENSITY
-  COMMERCIAL
-  OPEN SPACE

- POC: POINT OF CONCENTRATION
-  SUBBASIN NUMBER

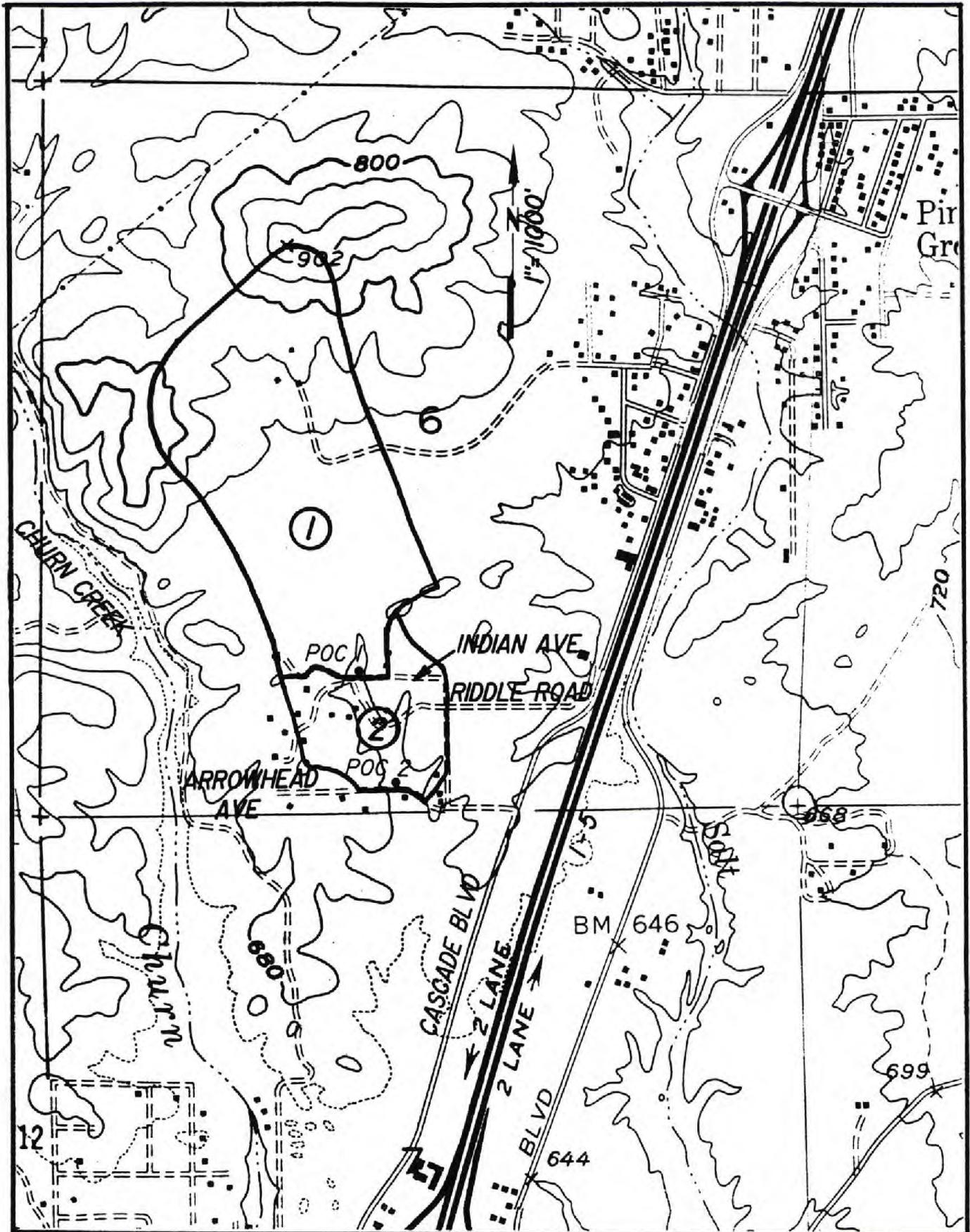
FIGURE 5
CITY OF SHASTA LAKE
PRELIMINARY DRAINAGE STUDY
SUMMIT CITY AREA
FUTURE LAND USE

PREPARED BY



SCALE: 1"=800'

JOB# 110.24



DATE

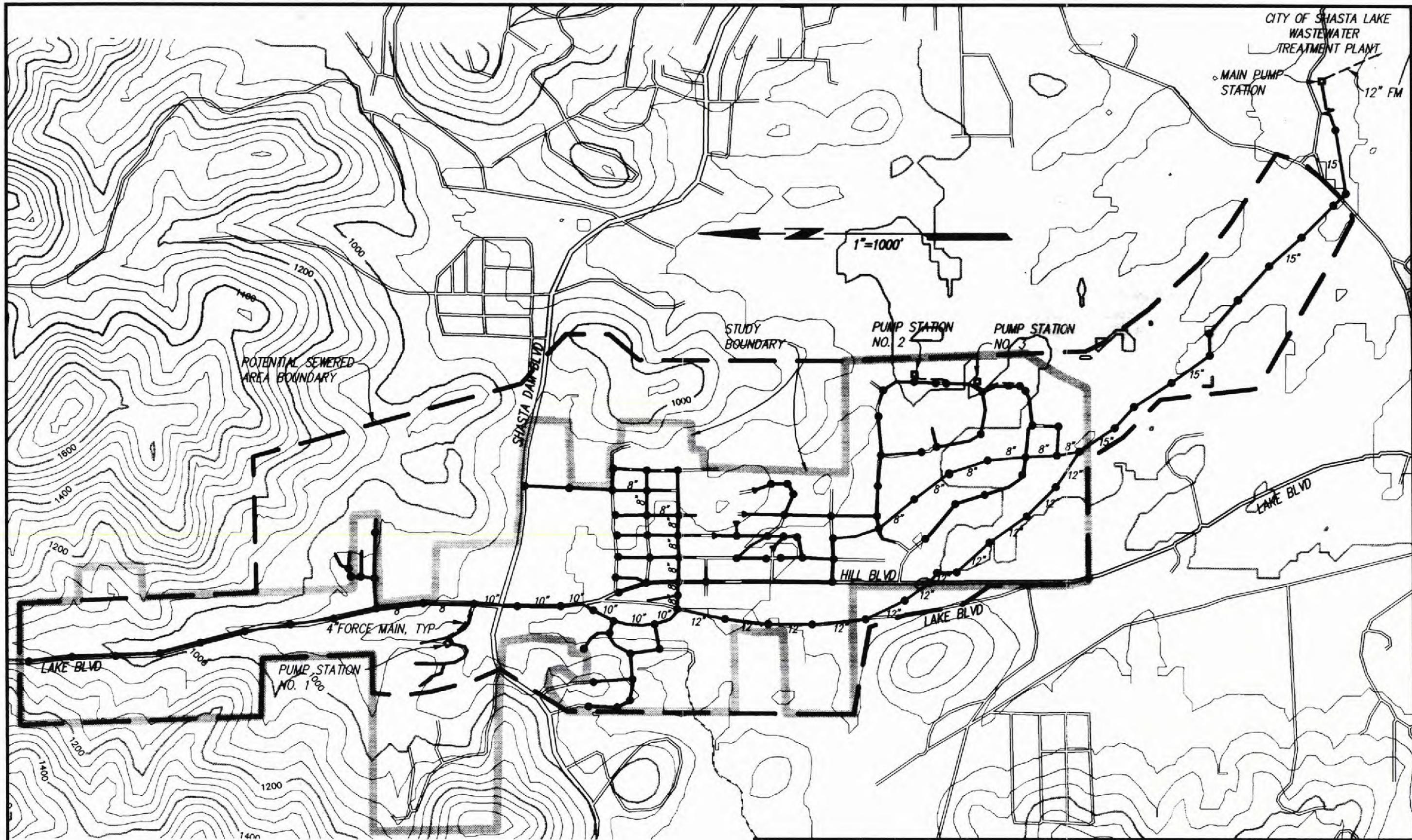
5/97



CITY OF SHASTA LAKE
PRELIMINARY DRAINAGE STUDY
RIDDLE ROAD AREA DRAINAGE AREAS

FIGURE 6

JOB # 110.24



CITY OF SHASTA LAKE
WASTEWATER
TREATMENT PLANT
MAIN PUMP
STATION

1"=1000'

POTENTIAL SEWERED
AREA BOUNDARY

STUDY
BOUNDARY

PUMP STATION
NO. 2

PUMP STATION
NO. 3

SHASTA DAM BLVD

LAKE BLVD

HILL BLVD

LAKE BLVD

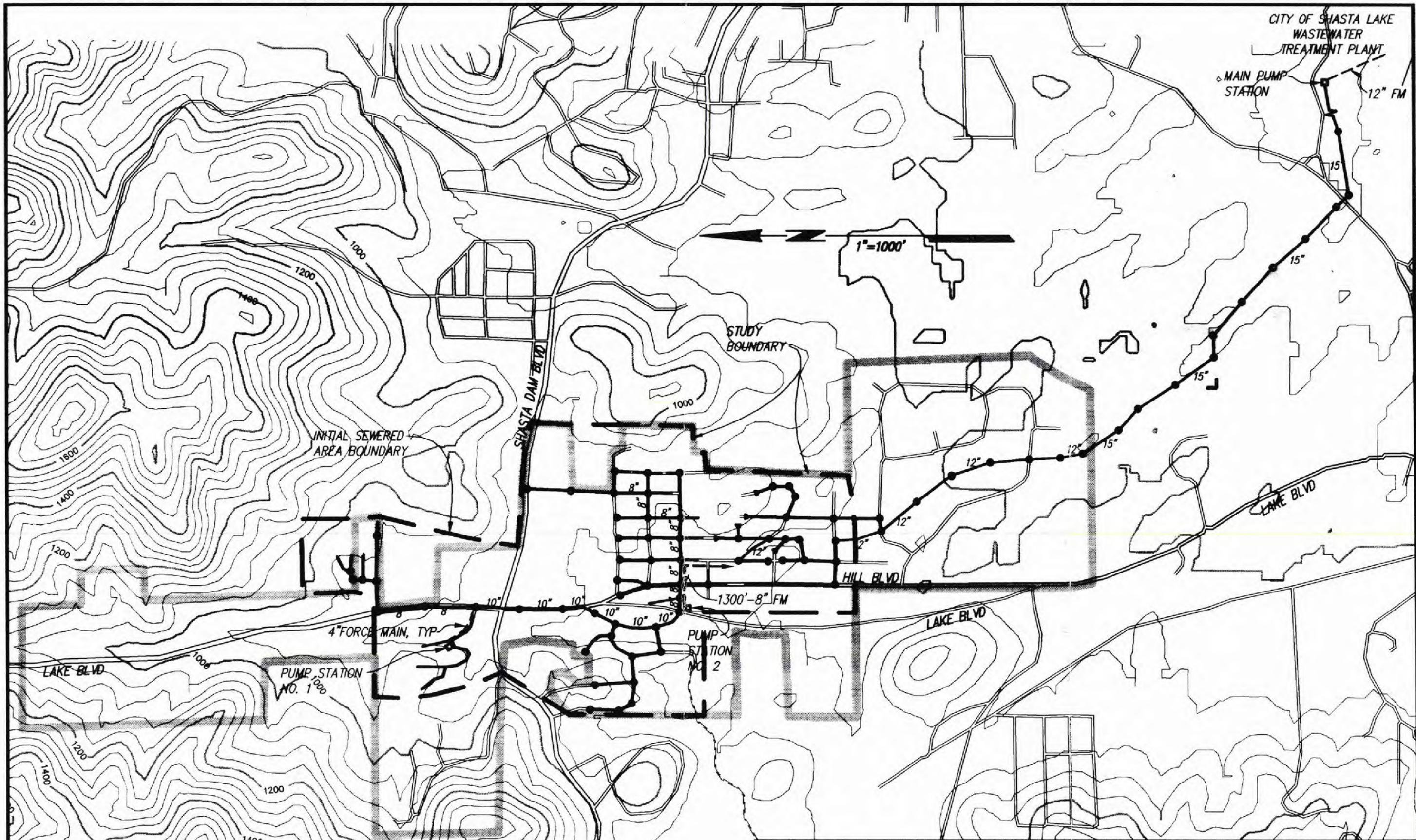
PUMP STATION
NO. 1

4" FORCE MAIN, TYP

PAOE
ENGINEERING
REDDING, CALIFORNIA

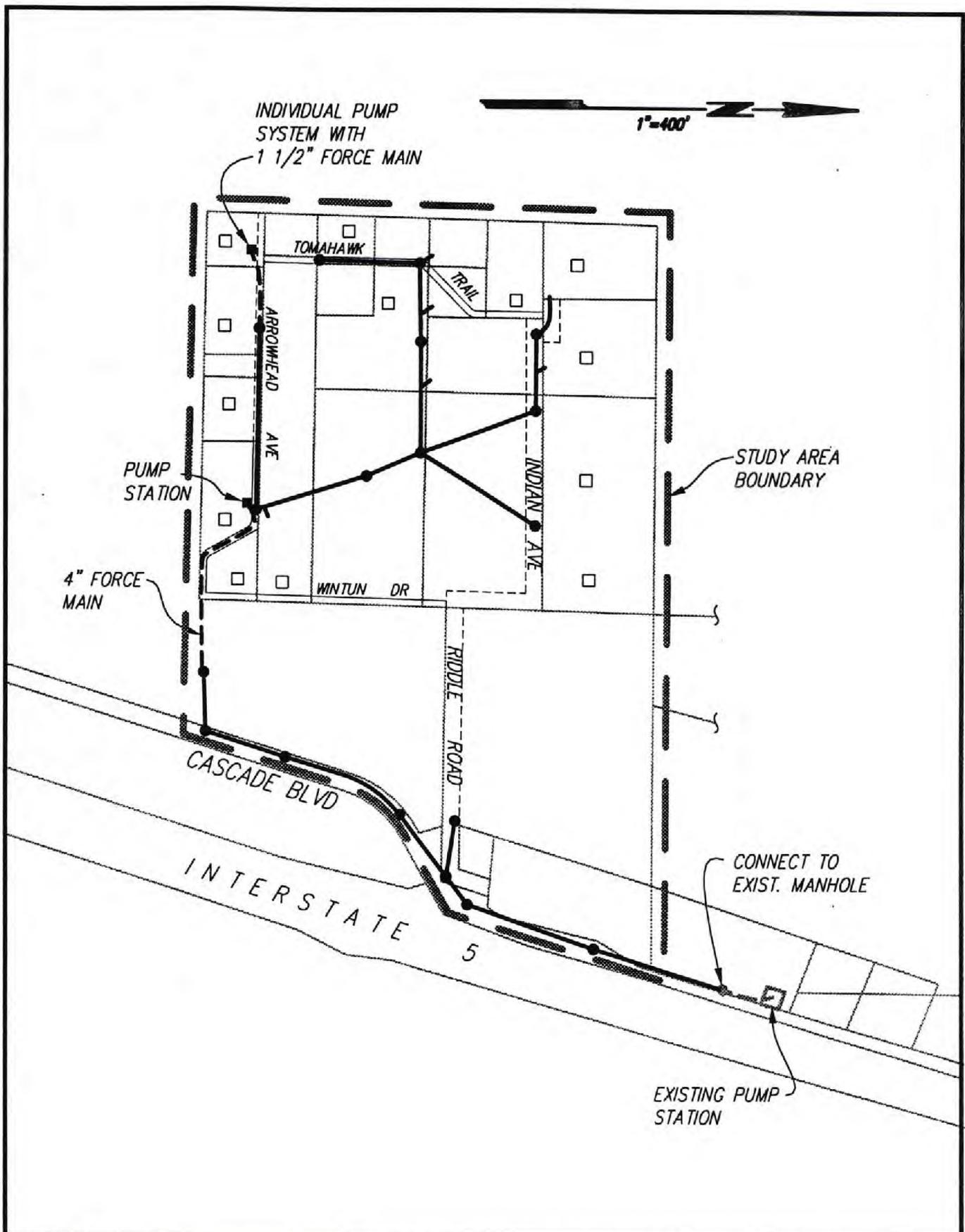
PRELIMINARY SEWAGE COLLECTION SYSTEM
LAYOUT TO SERVE MOST OF SUMMIT CITY AREA

FIGURE 7
DATE 05/27/97
JOB # 110.24



PRELIMINARY SEWAGE COLLECTION SYSTEM
LAYOUT TO SERVE HIGH DENSITY PORTION
OF SUMMIT CITY AREA

FIGURE 8
DATE 05/27/97
JOB # 110.24



| | | | |
|-------------------------|--|---|---|
| <p>DATE</p> <p>5/97</p> | <p>PACE </p> <p>ENGINEERING</p> <p>REDDING, CALIFORNIA</p> | <p>PRELIMINARY SEWAGE COLLECTION SYSTEM LAYOUT FOR RIDDLE ROAD AREA</p> | <p>FIGURE 9</p> <hr/> <p>JOB # 110.24</p> |
|-------------------------|--|---|---|

APPENDIX

APPENDIX A INCOME SURVEY

During the Infrastructure Needs Study, a questionnaire was mailed out to all of the parcels of land in the two study areas. An income survey form (see copy attached at the end of this appendix) was attached to the questionnaire and most of the people that returned the questionnaire also responded to the income survey questions.

SUMMIT CITY AREA RESPONSE:

There are a total of 578 parcels of land in the Summit City study area and it is estimated that only about 400 are occupied with either a residence or commercial building.

The following is a summary of the data obtained from the 163 residential property owners that responded to the survey:

| | <u>Number of Residences</u> | |
|---|---------------------------------|----------|
| • Income Level A | 37 | or 22.7% |
| • Income Level B | 91 | or 55.8% |
| • Income greater than Level B or no response | 35 | or 21.5% |
| • HEAD OF HOUSEHOLD BREAKDOWN | | |
| Own their home | 139 | or 85% |
| Rentals | 11 | or 7% |
| Females | 60 | |
| Over 62 | 66 | |
| Males | 122 | |
| White | 125 | |
| American Indian | 8 | |
| Spanish/Hispanic | 5 | |
| Handicapped | 20 | |

The Target Income Levels A and B represent 78.5 percent of the total responses which yields a standard deviation of 0.41. Based upon the sample size of 163 households the standard error is calculated at 3 percent, which is less than a standard error of 5 percent required for a valid random sampling process. Therefore, it appears that 78.5 percent of the Summit City area household incomes are equal to or less than the target income levels.

RIDDLE ROAD AREA RESPONSE:

There are a total of 24 parcels of land in the Riddle Road study area and it is estimated that only about 20 are occupied with either a residence or commercial building.

The following is a summary of the data obtained from the four residential property owners with improvements on their property that responded to the survey:

| | <u>Number of Residences</u> | |
|---|---------------------------------|--------|
| • Income Level A | 1 | or 25% |
| • Income Level B | 2 | or 50% |
| • Income greater than Level B or no response | 1 | or 25% |
| • HEAD OF HOUSEHOLD BREAKDOWN | | |
| Own their home | 1 | |
| Rentals | 2 | |
| Females | 0 | |
| Over 62 | 1 | |
| Males | 2 | |
| White | 0 | |
| American Indian | 1 | |
| Spanish/Hispanic | 0 | |
| Handicapped | 2 | |

The Target Income Levels A and B represent 75 percent of the total responses which yields a standard deviation of 0.43. Based upon the sample size of 4 households the standard error is

calculated at 22 percent, which is greater than a standard error of 5 percent required for a valid random samplings. Therefore, the questionnaire income survey is not considered to be a valid sampling. Because of the low number of households in the Riddle Road area, income information will need to be obtained from all households in order to have a valid survey.

**CITY OF SHASTA LAKE
SUMMIT CITY STUDY AREA
INCOME SURVEY**

1. How many persons live in your household? _____
2. Calculate the total income in the last 12 months or the income claimed in the most recent Federal income tax filing of all the persons who currently live in you household and check whether your income falls into Range A or B of the chart below. A B

(Total income includes: wages before deductions, public assistance, unemployment benefits, social security, pensions, alimony, child support, net income from owning or operating a farm or business, any other source of income received regularly.)

| NUMBER OF PESONS LIVING IN HOUSEHOLD | | | | | | | | |
|--------------------------------------|-------------------|-------------------|-------------------|-------------------|-------------------|-------------------|-------------------|-------------------|
| Income Range | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 |
| A | \$0-\$12,400 | \$0-\$14,150 | \$0-\$15,950 | \$0-\$17,700 | \$0-\$19,100 | \$0-\$20,550 | \$0-\$21,950 | \$0-\$23,350 |
| B | \$12,401-\$19,800 | \$14,151-\$22,550 | \$15,951-\$25,500 | \$17,701-\$28,300 | \$19,101-\$30,600 | \$20,551-\$32,850 | \$21,951-\$35,100 | \$23,351-\$37,400 |

3. Are your living quarters: Owned by you Rented
Name and address of owner: _____
4. If living quarters are owned, do all persons holding title to the property reside here? Yes No
5. Is the head of household: (Check all that apply)

| | | |
|---|---|--|
| <input type="checkbox"/> Female | <input type="checkbox"/> White | <input type="checkbox"/> American Indian or Alaskan Native |
| <input type="checkbox"/> Over 62 years of age | <input type="checkbox"/> Black | <input type="checkbox"/> Spanish/Hispanic |
| <input type="checkbox"/> Male | <input type="checkbox"/> Asian/Pacific Islander | |
| <input type="checkbox"/> Other | | |
6. Do any physically or mentally handicapped people reside in this household? Yes No

NOTE: Replies to this income survey are confidential. Confidentiality is protected by not including names on the form. We need a high response to have a chance of getting funded. Addresses are on the "Infrastructure Needs Study" questionnaire so we can make follow-up contacts if no questionnaire is received and we need more questionnaires returned. When we get your questionnaire and income survey in the mail, we will check off your address and remove the income survey from the questionnaire. No identifying information will be kept with the income surveys and the income surveys will be tallied as a group.

Any questions or comments, please contact John Pedri, City Engineer, at 916/275-7422.